

**TAUNTON RIVER BASIN  
TAUNTON, MASSACHUSETTS**

**PROSPECT HILL RESERVOIR  
MA 00811**

# **PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM**

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**DEPARTMENT OF THE ARMY  
NEW ENGLAND DIVISION, CORPS OF ENGINEERS  
WALTHAM, MASS. 02154**

**APRIL 1979**

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number)  The dam measures 604 ft. by 274 ft. It is impounded by a rectangular, concrete lined earth embankment. It provides storage and equalizing pressure for the Taunton Water supply system. There is need for maintenance and monitoring to assure the continued performance of this reservoir and embankment. The reservoir and embankment are generally in good condition and well maintained.		



DEPARTMENT OF THE ARMY  
NEW ENGLAND DIVISION, CORPS OF ENGINEERS  
424 TRAPELO ROAD  
WALTHAM, MASSACHUSETTS 02154

REPLY TO  
ATTENTION OF:  
NEDED

JUN 18 1979

Honorable Edward J. King  
Governor of the Commonwealth of  
Massachusetts  
State House  
Boston, Massachusetts 02133

Dear Governor King:

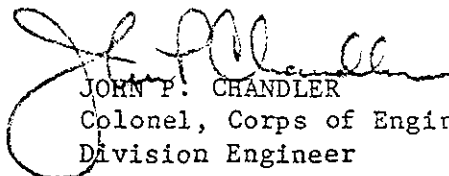
I am forwarding to you a copy of the Prospect Hill Reservoir Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Quality Engineering, the cooperating agency for the Commonwealth of Massachusetts. In addition, a copy of the report has also been furnished the owner, Taunton Water Works, City Hall, Room 9, Taunton, Massachusetts 02780.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Quality Engineering for your cooperation in carrying out this program.

Sincerely yours,

  
JOHN P. CHANDLER  
Colonel, Corps of Engineers  
Division Engineer

Incl  
As stated

PROSPECT HILL RESERVOIR

MA 00811

TAUNTON RIVER BASIN  
TAUNTON, MASSACHUSETTS

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION  
PROGRAM



NATIONAL DAM INSPECTION  
PROGRAM  
PHASE I INSPECTION REPORT  
BRIEF ASSESSMENT

Identification No.: MA00811

Name of Dam: Prospect Hill Reservoir

Town: Taunton

County and State: Bristol County, Massachusetts

Stream: None - surface drainage to tributaries of  
Taunton River

Date of Inspection: November 30, 1978

Prospect Hill Reservoir is impounded by a rectangular, concrete-lined earth embankment. The reservoir, which was built in 1956, measures 604 feet by 274 feet. The embankment is about 1,200 feet long with a maximum height of 25 feet and side slopes at 2:1 (horizontal to vertical). The concrete rim of the reservoir is at elevation (El) 194.0, and the crest of the embankment varies from El 192.9 to 193.2.

Prospect Hill Reservoir provides storage and equalizing pressure for the Taunton water supply system. Two pipelines lead to a gate house located on the south side of the reservoir. These lines branch into four 20-inch conduits which carry flow into and out of the reservoir. Water is pumped into the reservoir at certain times of the day, and then allowed to flow out on demand. The operational high water level is at El 192.0, resulting in a maximum storage capacity of 22 million gallons (mil. gal.).

There are two high-level overflow drains and two low-level drains which connect to a single 12-inch outlet conduit. The two high-level outlets are 12-inch overflow pipes set vertically with rims at El 192.5. The two low-level outlets are 12-inch drain pipes with inverts at El 162.0. The outlet conduit

PROSPECT HILL RESERVOIR

leads 200 feet downhill to an overflow pond with 1 mil. gal. capacity. The overflow pond has no outlet and water overflows the embankment into a ditch along Prospect Hill Street and eventually to Prospect Hill Pond.

There is a need for maintenance and monitoring to assure the continued performance of this reservoir and embankment. This conclusion is based upon the visual inspection at the site, the available engineering data, and past performance history. According to the Corps of Engineers guidelines on classification of hazard potential, the reservoir has been placed in the "significant" hazard category.

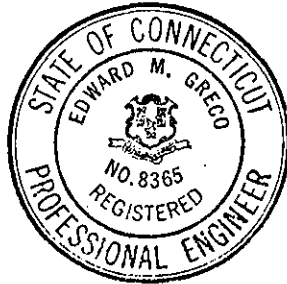
The reservoir and embankment are generally in good condition and well maintained. The following deficiencies were observed at the site: growth of trees on the slopes of the embankment, slight leakage from some joints on the conduits, and leakage from a gate valve into the outlet conduit. It was also noted that a greater than expected amount of seepage is occurring in the underdrain system.

Hydraulic analyses indicate that the two 12-inch overflow drains can discharge an estimated flow of 15.1 cubic feet per second (cfs) with the water surface at El 194.0 which is the top of the rim of the reservoir. An outflow test flood of 7.7 cfs (one-half the probable maximum flood (PMF)) at El 192.9 will not overflow the reservoir. Under normal operating conditions, the reservoir can discharge 100 percent of the test flood without overflowing the rim.

It is recommended that the Owner selectively remove trees from the embankment, repair leakage from the conduits, repair the gate valve on the outlet, and monitor and measure the seepage from the underdrain system. The Owner should also implement a regular program of technical inspections.

#### PROSPECT HILL RESERVOIR

The remedial measures outlined above and in Section 7 should be implemented by the Owner within a period of 2 years after receipt of this Phase I Inspection Report.



A handwritten signature in cursive script, reading "Edward M. Greco".

Edward M. Greco, P.E.  
Project Manager  
Metcalf & Eddy, Inc.

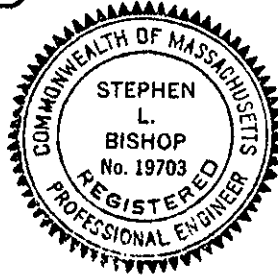
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No. 08365

Approved by:

A handwritten signature in cursive script, reading "Stephen L. Bishop".

Stephen L. Bishop, P.E.  
Vice President  
Metcalf & Eddy, Inc.

Massachusetts Registration  
No. 19703



PROSPECT HILL RESERVOIR

This Phase I Inspection Report on Prospect Hill Reservoir has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

*Joseph A. McElroy*

JOSEPH A. MCELROY, MEMBER  
Foundation & Materials Branch  
Engineering Division

*Carney M. Terzian*

CARNEY M. TERZIAN, MEMBER  
Design Branch  
Engineering Division

*Joseph W. Finegan, Jr.*

JOSEPH W. FINEGAN, JR., CHAIRMAN  
Chief, Reservoir Control Center  
Water Control Branch  
Engineering Division

APPROVAL RECOMMENDED:

*Joe B. Fryar*

JOE B. FRYAR  
Chief, Engineering Division

## PREFACE

This report is prepared under guidance contained in Recommended Guidelines for Safety Inspection of Dams, for a Phase I Investigation. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test Flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general conditions and the downstream damage potential.

PROSPECT HILL RESERVOIR

## TABLE OF CONTENTS

	<u>Page</u>
BRIEF ASSESSMENT	
PREFACE	
OVERVIEW PHOTO	iii
LOCATION MAP	iv
REPORT	
SECTION 1 - PROJECT INFORMATION	1
1.1 General	1
1.2 Description of Project	1
1.3 Pertinent Data	6
SECTION 2 - ENGINEERING DATA	10
2.1 General	10
2.2 Construction Records	10
2.3 Operating Records	10
2.4 Evaluation of Data	11
SECTION 3 - VISUAL INSPECTION	12
3.1 Findings	12
3.2 Evaluation	14
SECTION 4 - OPERATING PROCEDURES	15
4.1 Procedures	15
4.2 Maintenance of Reservoir and Embankment	15
4.3 Maintenance of Operating Facilities	15
4.4 Description of Any Warning System in Effect	15
4.5 Evaluation	15
SECTION 5 - HYDRAULIC/HYDROLOGIC	16
5.1 Evaluation of Features	16

PROSPECT HILL RESERVOIR

## TABLE OF CONTENTS (Continued)

	<u>Page</u>
SECTION 6 - STRUCTURAL STABILITY	19
6.1 Evaluation of Structural Stability	19
SECTION 7 - ASSESSMENT, RECOMMENDATIONS, AND REMEDIAL MEASURES	21
7.1 Dam Assessment	21
7.2 Recommendations	22
7.3 Remedial Measures	22
7.4 Alternatives	22

## APPENDIXES

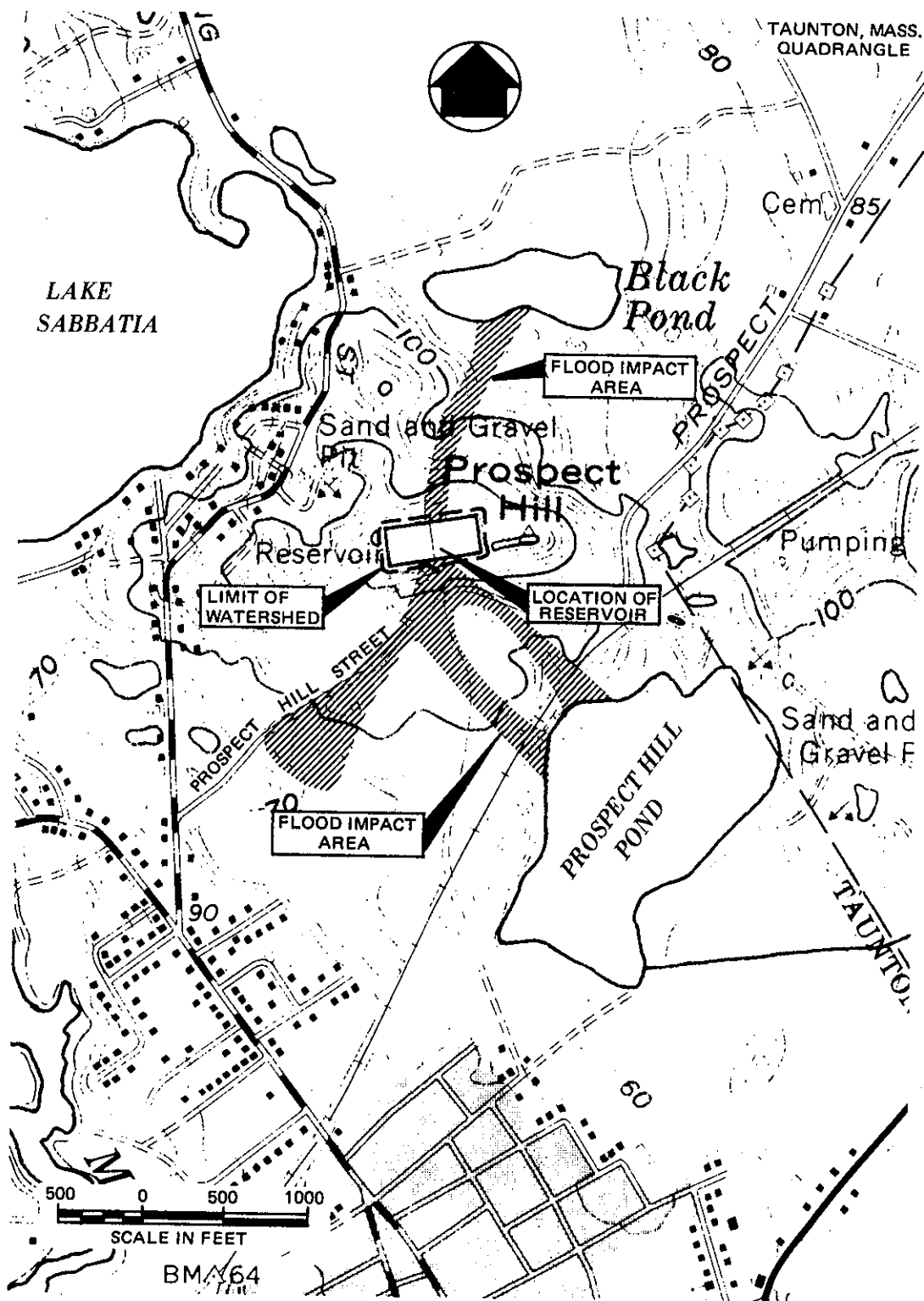
APPENDIX A - PERIODIC INSPECTION CHECKLIST
APPENDIX B - PLANS OF RESERVOIR
APPENDIX C - PHOTOGRAPHS
APPENDIX D - HYDROLOGIC AND HYDRAULIC COMPUTATIONS
APPENDIX E - INFORMATION AS CONTAINED IN THE NATIONAL INVENTORY OF DAMS



OVERVIEW  
PROSPECT HILL RESERVOIR  
TAUNTON, MASSACHUSETTS







LOCATION MAP - PROSPECT HILL RESERVOIR

NATIONAL DAM INSPECTION  
PROGRAM

PHASE I INSPECTION REPORT

PROSPECT HILL RESERVOIR

SECTION 1

PROJECT INFORMATION

1.1 General

- a. Authority. Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of dam inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Metcalf & Eddy, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of Massachusetts. Contract No. DACW 33-79-C-0016, dated November 28, 1978, has been assigned by the Corps of Engineers for this work.
- b. Purpose:
  - (1) Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
  - (2) Encourage and assist the States to initiate quickly effective dam safety programs for non-Federal dams.
  - (3) Update, verify and complete the National Inventory of Dams.

1.2 Description of Project

- a. Location. The reservoir is located on Prospect Hill in the City of Taunton, Bristol County, Massachusetts (see Location Map).

PROSPECT HILL RESERVOIR

b. Description of Dam and Appurtenances.

Prospect Hill Reservoir is impounded by a rectangular, concrete-lined earth embankment (see Figures B-1 through B-4). The reservoir is 604 feet long by 274 feet wide and is separated into two sections by a concrete wall. The reservoir has a flat bottom with inside slopes of 2:1 (horizontal:vertical), and a 1-foot high, vertical concrete wall around the rim. Two 6-inch underdrains are located beneath the center of the reservoir. Contract drawings show that the top of the rim of the reservoir is at El 194.0 and the bottom is at El 167.0. The operational high water level of the reservoir is at El 192.0

The earth embankment is about 1,200 feet long and encompasses most of the reservoir (see Figure B-2). Sections of the reservoir were excavated into natural ground at the northwest corner and for portions of the north and east sides. The height of the embankment varies, depending on the elevation of the natural ground surface, up to a maximum height of about 25 feet at the gate house. The crest of the embankment is about 7 feet wide and varies from El 192.9 to 193.2. The outside slope is at 2:1 and is covered with grass and some planted pine trees. A typical embankment section is shown on Figure B-3.

Prospect Hill Reservoir provides storage and equalizing pressure for the Taunton water supply system. Controls for the conduits transmitting flow into and out of the reservoir are located in a combined screen, chlorination and gate house on the south side of the reservoir (see Figures B-5 and B-6). Two water supply pipelines, 20 inches and 24 inches in diameter, lead into the gate house. These branch into two 20-inch inflow and two 20-inch outflow cast-iron conduits. The inflow conduits lead from the gate house, through the embankment and diagonally across the floor of the reservoir to the northwest and northeast corners (see Figure B-2). The outflow conduits lead from the floor of the reservoir to the screen house, where 20-inch square sluice gates control flow into two sections. There is also a 20-inch square

PROSPECT HILL RESERVOIR

sluice gate regulating flow between the two sections. Two 20-inch outflow conduits then lead from the screen house into the gate house where they connect into the water supply pipelines. The direction of flow through the conduits is controlled by check valves in the gate house (see Figure B-5). Each conduit also has a manually-operated gate valve. The invert of the conduits is shown on the drawings at El 167.0 on the bottom of the reservoir and at El 165.9 in the gate house.

Two high-level overflow drains and two low-level drains connect to a single 12-inch diameter conduit which conveys flow from the gate house to an overflow pond. The high-level outlets are 12-inch, cast-iron overflow pipes located in each section of the screen house. The two high-level outlets are set vertically and are shown on the drawings with rims at El 192.5. This elevation is 1/2 foot above the operational high water level. The low-level outlets are two 12-inch, cast-iron drain pipes leading from the bottom of each section of the screen house. They are controlled by hand-operated gate valves in the gate house and are shown on the drawings with inverts at El 162.0. The four 12-inch outlets connect into a 12-inch, cast-iron outlet conduit which discharges into the overflow pond. The invert of the outlet conduit is at El 130.5 where it enters the pond.

About 200 feet southwest of the gate house is a 1 mil. gal. capacity overflow pond. The pond is about 210 feet long, 90 feet wide, and 6 feet deep. An earth dike has been constructed around the southern half of the pond. The crest of the dike varies from El 135.5 to 136.0. The sides slope at 2:1 (see Sections Through Blow-off Basin, Figure B-3). The overflow pond receives water from the 12-inch outlet conduit and a 6-inch under-drain from beneath the reservoir lining. There is no outlet for the pond and, if it becomes full, water overflows the dike and flows in an open ditch along Prospect Hill Street.

- c. Size Classification. Prospect Hill Reservoir is classified in the "small" category since the embankment has a maximum height of 25 feet, and the reservoir has a maximum storage capacity of 77 acre-feet.
- d. Hazard Classification. The reservoir is located on top of a hill about 3,000 feet north of thickly developed areas of Taunton. The area on the north side of Prospect Hill is undeveloped and leads to a broad lowland area where Black Pond and a swamp are located. The area on the south side of Prospect Hill is mostly undeveloped and leads to a swamp and Prospect Hill Pond. There is light residential development along Prospect Hill Street, beginning about 600 feet southwest of the reservoir.

In the event of complete failure of the reservoir, it is unlikely that extensive property damage would occur or that more than a few lives would be lost. Accordingly, the dam has been placed in the "significant" hazard category.

- e. Ownership. The reservoir is owned by the Taunton Water Works, City Hall, Room 9, Taunton, Massachusetts 02780. Mr. Joseph Sousa, Superintendent, (telephone 824-5859) granted permission to enter the property and inspect the reservoir.
- f. Operators. The reservoir is operated by personnel from the Taunton Water Works. The gate valves on the inflow and outflow conduits as well as the slide gates in the screen house are kept open. However, personnel inspect the gate house and reservoir at least once each day.
- g. Purpose of Dam. The reservoir is used to store water and equalize pressure for the City of Taunton water supply system. Water is pumped into the reservoir during periods of low demand or when the water level is low. Water is used from the reservoir during periods of high demand. The capacity of the reservoir at the operational high water level (El 192.0) is 22 mil. gal.

#### PROSPECT HILL RESERVOIR

- h. Design and Construction History. The reservoir, embankment and overflow pond were constructed in 1956 by R. Zoppo Construction Co. of Norwood, Massachusetts. The facility was designed by Fay, Spofford & Thorndike, Inc. of Boston, Massachusetts. The construction specifications refer to ASTM standards for concrete, cement, reinforcing and other materials. Material for the earth embankment was obtained from cut sections around the reservoir. This material was reportedly sandy gravel, which agrees with what is shown in the construction photographs. The foundation beneath the reservoir is also sandy gravel. Borings were taken prior to construction, although the logs are no longer available. Fay, Spofford & Thorndike, Inc. provided full-time inspection during construction of the project. The structures are built essentially as shown on the drawings, and no significant post-construction changes have been made.

A plastic floating cover has been designed for the reservoir and is expected to be constructed in the summer of 1979. The cover will rise and fall with the fluctuating water level and will help to keep debris out of the water. The cover was designed by Shevalier Engineers of Raynham, Massachusetts.

- i. Normal Operating Procedures. Pumps transmitting water to the reservoir are operated to maintain a water level at about El 192.0. The maximum pumping rate is 13.9 cfs from the two pumps located at an off-site pumping station. Personnel at the pumping station monitor the reservoir level and pumping rates 24 hours a day.

The gates valves for the inflow and outflow conduits and the sluice gates in the screen house are kept open. The facilities are checked daily by personnel from Taunton Water Works. All gates and fittings are greased and checked every six months. Occasionally, when debris in the reservoir needs to be cleaned out, one section is drained using the low-level outlet. One side was cleaned three years ago and the other cleaned eight years

PROSPECT HILL RESERVOIR

ago. Doors into the gate house and screen house are kept locked and the access road from Prospect Hill Street has a locked gate.

### 1.3 Pertinent Data

- a. Drainage Area. The reservoir is located on top of a hill with its rim above the surrounding ground. Surface runoff drains away from the reservoir to the north and south. The drainage area, therefore, consists only of the surface area of the reservoir (see Location Map). This drainage area (reservoir area) is 3.77 acres (0.006 square miles).
- b. Discharge. Normal discharge is through two 20-inch conduits which extend from the bottom of the reservoir through the southern embankment and into a screen house. These conduits are about 65 feet long and have an upstream invert at El 167.0. There are two sluice gates at the screen house which control flow through the conduits. From the screen house, two 20-inch conduits carry flow through the gate house and into 20-inch and 24-inch water supply pipelines. The invert of the conduits is at El 165.9 in the gate house. Flow is normally controlled by check valves, but hand-operated gate valves are also present in the gate house.

If the water in the reservoir rises above the normal high level, two 12-inch overflow drains in the screen house would carry flow to a 12-inch outlet conduit in the gate house. The rims of the overflow drains are at El 192.5. The outlet conduit leads 200 feet downhill and into an overflow pond with 1 mil. gal. capacity. The downstream invert of the outlet conduit is at El 130.5. If the pond becomes full, water overflows into a 2-foot wide ditch along Prospect Hill Street. This ditch leads to a culvert beneath the road and into a swamp which drains into Prospect Hill Pond.

The 12-inch overflow drains can discharge an estimated 15.1 cfs with the water surface at El 194.0 which is the top of the rim of the

PROSPECT HILL RESERVOIR

reservoir. Under normal operating conditions, an outflow test flood (one-half the PMF) of 7.7 cfs at El 192.9 will not overflow the rim of the reservoir.

The reservoir was built in 1956 and has never reportedly overflowed. The water level, which is controlled by pumping, is recorded in the pumping station. The maximum reservoir level is recorded to be 192.5.

c. Elevation (feet above Mean Sea Level (MSL)).

A benchmark was established at El 194.0 on the top of the concrete wall dividing the reservoir. This elevation was shown on drawings of the reservoir dated 1954 (see Figure B-3).

- (1) Top dam: 193.9 to 194.0 - top of concrete wall  
192.9 to 193.2 - top of earth embankment
- (2) Test flood pool: 192.9
- (3) Design surcharge (1954 design): 192.5  
rims of overflow pipes
- (4) Full flood control pool: Not Applicable (N/A)
- (5) Recreation pool (operational pool): 192.0
- (6) Spillway crest: None (overflow drains at El 192.5)
- (7) Upstream portal invert diversion tunnel: None
- (8) Stream bed at centerline of dam: N/A
- (9) Maximum tailwater: N/A

d. Reservoir

- (1) Length of maximum pool: 300 feet
- (2) Length of recreation pool (operational pool): 300 feet

PROSPECT HILL RESERVOIR



(3) Length of flood control pool: N/A

e. Storage (acre-feet)

(1) Test flood surcharge (net): 3.4 at El 192.9

(2) Top of dam: 77 at El 194.0

(3) Flood control pool: N/A

(4) Recreation pool (operational pool): 70

(5) Spillway crest: N/A

f. Reservoir Surface (acres)

\*(1) Top dam: 3.8

\*(2) Test flood pool: 3.8

(3) Flood-control pool: N/A

(4) Recreation pool (operational pool): 3.8

(5) Spillway crest: N/A

g. Dam (earth embankment and reservoir)

(1) Type: earthfill

(2) Length: 1,200 feet

(3) Height: maximum 25 feet

(4) Top width: 7 feet

(5) Side slopes: 2:1

(6) Zoning: None

(7) Impervious core: None

(8) Cutoff: concrete lining on inside slopes

(9) Grout curtain: None

\*Based on the assumption that the surface area will not increase significantly with changes in reservoir elevation from 192.0 to 192.9.

PROSPECT HILL RESERVOIR

- i. Spillway. There is no spillway at this site. Normal discharge is carried by two 20-inch conduits leading to 20-inch and 24-inch water supply mains. Overflow and drawdown are carried by a 12-inch outlet conduit leading to an overflow pond.
- j. Regulating Outlets. Under normal conditions, the water level is regulated by two water supply pipelines which lead into and out of the reservoir. These pipelines split into two 20-inch inflow and two 20-inch outflow conduits in the gate house. Check valves on the conduits direct the flow, and gate valves are present to stop flow when necessary. The invert of the conduits in the gate house is at El 165.9.

Water in the reservoir can be drawn down through a 12-inch, cast-iron conduit which leads from the gate house to an overflow pond about 200 feet downhill. Flow enters the conduit through two 12-inch gated low-level drains leading out of the screen house. The invert of the pipes is at El 162.0 at the screen house. The invert of the outlet conduit is at El 130.5 in the overflow pond.

SECTION 2  
ENGINEERING DATA

- 2.1 General. There are 45 sheets of contract drawings dated September, 1954, available from Taunton Water Works and from Zoppo Construction Co. These drawings show plans and sections of the reservoir, gate house, overflow pond, and appurtenances. Copies of five selected drawings are included in Appendix B (Figures B-2 through B-6). Copies of the construction specifications and a set of 8-inch by 10-inch photographs taken during construction are available from Fay, Spofford & Thorndike, Inc.

Drawings showing the design of the proposed floating cover for the reservoir are available from Shevalier Engineers of Raynham, Massachusetts. No hydraulic computations or construction records are available from the Owner, County or State agencies relative to the design and construction of this reservoir.

We acknowledge the assistance and cooperation of the following people: Messrs. John Hannon and Joseph Iagallo of the Massachusetts Division of Waterways; Mr. Albert Lounsbury of the Massachusetts Department of Public Works; Messrs. Joseph Sousa and Manuel Souza of the Taunton Water Works; Mr. Clifford Mansfield of Fay, Spofford & Thorndike, Inc.; and Mr. Tom Zoppo of Zoppo Construction Co.

- 2.2 Construction Records. The only construction records available are the contract drawings, specifications and photographs referred to in Section 2.1. The drawings are noted as being revised to as-built drawings in July, 1956. Copies of five selected sheets are included in Appendix B.
- 2.3 Operating Records. Records of the water level, pumping rates and hours of pumping are kept by personnel at the pumping station. Personnel are at the station 24 hours a day and manually record the pumping data and water level every four hours.

## 2.4 Evaluation

- a. Availability. There are as-built drawings, construction specifications and operating records available for the reservoir.
- b. Adequacy. The lack of hydraulic and structural computations and detailed construction records did not allow for a definitive review. Therefore, the evaluation of the adequacy of the reservoir is based on review of available drawings, visual inspection, past performance history, and engineering judgment.
- c. Validity. Comparison of the as-built drawings with the field survey conducted during the Phase I inspection indicates that the available information is valid.

SECTION 3  
VISUAL INSPECTION

3.1 Findings

- a. General. The Phase I Inspection of the Prospect Hill Reservoir was performed on November 30, 1978. A copy of the inspection checklist is included in Appendix A. A brief inspection of the reservoir was made by personnel from the Department of Public Works in January, 1978. The only deficiency noted in the report was the growth of pine trees on the outside slopes of the embankment.
- b. Dam (reservoir and embankment). Prospect Hill Reservoir is a rectangular, concrete-lined structure with an earth embankment around most of its perimeter. The reservoir and embankment are generally in good condition. The visible concrete lining is slightly discolored and eroded below the operational high water level. Joint filler is missing from most of the expansion joints between the concrete slabs. Two 6-inch underdrains are located beneath the reservoir lining. Personnel from the water company reported that leakage through the lining is greater than expected, based on observed flows from the underdrains. A 2-foot high, corrugated asbestos fence, which serves as a rodent barrier, is located on the rim of the reservoir. The fence has been repaired in several areas due to vandalism.

The embankment is constructed of random earth fill and serves to support the sides of the reservoir. The outside slope is at 2:1 and is covered with grass; some areas were planted with pine trees for erosion control. Minor surface erosion was noted in places. No seepage or settlement of the embankment was visible.

- c. Appurtenant Structures. There is no spillway at this site. Conduits leading into and out of the reservoir are controlled by a combined screen house and gate house located on the

PROSPECT HILL RESERVOIR

south side of the reservoir. The building is made of concrete and is in good condition. The conduits are made of cast-iron with bell and spigot joints. Minor staining and leakage was observed at some joints. Seepage and staining was also visible where conduits extend from the wall in the screen house to the gate house.

About 200 feet below the gate house is an overflow pond constructed with a bermed earth dike around about half of its perimeter. The pond receives flow from a 6-inch underdrain and a 12-inch outlet conduit. There is no outlet for the pond, and water was slightly overflowing the dike at the time of inspection. This is mainly due to discharge from the underdrain. However, the outlet conduit is also reportedly discharging due to leakage from a gate valve. The crest and slope of the dike are covered with grass. There are also two small trees growing on the crest. The slope and crest are locally eroded where water has overtopped the dike, and the water is flowing downhill in a ditch along the toe of the slope. There is no riprap protection for the slope of the dike or the sides of the ditch along Prospect Hill Street.

- d. Reservoir Area. The reservoir is located on the top of a hill and the drainage area is limited to the surface area of the reservoir. Prospect Hill, which is generally undeveloped and wooded, has a maximum elevation at about El 200 with steep slopes of 12 to 25 percent. A sand and gravel pit is located northwest of the reservoir about halfway down the hill. Some residential development occurs near the bottom of the hill to the west and southwest of the reservoir.
- e. Downstream Channel. There is no discharge channel or stream at this site. Normal discharge flows into two water supply pipelines leading out of the gate house. Overflow and drawdown is transmitted to a pond downhill from the gate house. Water overflowing the pond discharges downhill along Prospect Hill Street in a 2-foot wide, unlined, open ditch for a distance of about 1,200 feet. Flow in

the ditch enters a culvert beneath the road which leads to a broad, swampy area draining into Prospect Hill Pond.

In the event of failure of the reservoir and embankment, water would flow downhill, either toward Prospect Hill Pond if the break occurs on the south side of the reservoir, or towards Black Pond if the failure occurs on the north side of the reservoir (see flood impact area shown on the Location Map).

- 3.2 Evaluation. The above findings indicate that the reservoir is in good condition. The facility is generally well maintained, however, there are some items which require attention. Recommended measures to improve these conditions are stated in Section 7.3.

## SECTION 4

### OPERATING PROCEDURES

- 4.1 Procedures. Pumps are operated from an off-site pumping station to maintain the reservoir pool at El 192.0. Personnel at the pumping station monitor the water level and pumping rates 24 hours a day. Two pumps are used, at a combined maximum flow of 13.9 cfs (9 mgd).

The gates of the conduits leading into and out of the reservoir and the sluice gates in the screen house are kept open. The gate valves are checked every six months. The reservoir and appurtenant structures are visually inspected daily by personnel from the Taunton Water Works. The access gate, screen house and gate house are kept locked. A daily record of the water level is maintained by personnel at the water treatment plant.

- 4.2 Maintenance of Reservoir and Embankment. The reservoir and embankment are well maintained. However, pine trees are growing on areas of the embankment slope.
- 4.3 Maintenance of Operating Facilities. The screen house and gate house are well maintained. However, leakage is occurring from some joints on the piping and from a gate on the outlet conduit.
- 4.4 Description of Any Warning System in Effect. The water level in the reservoir is continuously recorded on a chart and read every four hours at the pumping station. If the water level exceeds El 192.0, the pumps are shut down. Personnel are at the pumping station 24 hours a day.
- 4.5 Evaluation. There is a regular program of maintenance inspections and surveillance for Prospect Hill Reservoir. Technical inspections should be conducted on a regular basis, as recommended in Section 7.3.

PROSPECT HILL RESERVOIR



## SECTION 5

### HYDRAULIC/HYDROLOGIC

#### 5.1 Evaluation of Features

- a. General. Prospect Hill Reservoir is located on top of a hill and provides equalizing pressure and storage for a city water supply. The drainage area for the reservoir consists only of its surface area. The reservoir is a rectangular, concrete-lined structure with a rim at El 194.0. An earth embankment supports the sides of the reservoir. The embankment ties into the natural hillside and varies in height to a maximum of 25 feet. The outside slopes are at 2:1, and the crest is at about El 193.0.

The reservoir level is controlled by pumping at a nominal maximum rate of 9 mgd (13.9 cfs). The pool level is monitored at the pumping station, and the pumps are shut down when the water level reaches El 192.0. Overflow is through two ungated, 12-inch drains, one for each half of the reservoir. These drains have rims at El 192.5. The overflow drains lead to a single 12-inch outlet conduit.

- b. Design Data. There are no hydraulic or hydrologic computations available for the design of the reservoir.
- c. Experience Data. The water level is continuously recorded on a chart and manually recorded every four hours. The maximum recorded water level was at El 192.5. The reservoir has not overflowed since its construction in 1956.
- d. Visual Observations. The reservoir and embankment are generally well maintained. The facility is inspected daily. The overflow pipes are contained in a screen house. The water level in the reservoir is monitored 24 hours a day by personnel in the pumping station.

PROSPECT HILL RESERVOIR

A detailed discussion of the condition of the reservoir and embankment is presented in Section 3, Visual Inspection.

- e. Test Flood Analysis. According to the Corps of Engineers' guidelines, the reservoir has been placed in the "small" size category and in the "significant" hazard category.

The Test Flood (one-half the PMF) inflow to 3.8 acres of reservoir consists of direct precipitation of 9.5 inches in six hours, assuming no losses. The Test Flood analysis consisted of determining the maximum rise in water level due to this rainfall and evaluating the effect of uncontrolled pumping. The analysis is based on a pool level starting at El 192.5 (rim of overflow drains) and assumes no outflow into the water supply pipelines.

Hydraulic analyses indicate that the overflow drains can discharge an estimated flow of 15.1 cfs when the reservoir level is at El 194.0 which is the top of the rim. The Test Flood produces a maximum outflow of 7.7 cfs with the reservoir at El 192.9. If in addition to the rainfall, the pumps continued to supply 13.9 cfs during the Test Flood, the reservoir would be slightly overtopped.

Overtopping of the reservoir will not occur under the Test Flood unless both pumps are allowed to continue pumping. Since the pool is normally maintained at El 192.0, which is one-half foot lower than the rims of the overflow pipes, the probability of overtopping is very low.

The reservoir has two 12-inch, low-level drains used to lower the water level for maintenance. Drawdown of the reservoir in anticipation of a storm could be achieved by shutting off the pumps and allowing normal outflow by demand to lower the pool.

- f. Dam Failure Analysis. The peak discharge rate due to failure was calculated to be 23,600 cfs for a 100-foot long section of the embankment. The total failure head of 27 feet was based on

PROSPECT HILL RESERVOIR

a pond level of 194.0, which is the rim elevation of the reservoir, and an elevation of 167.0, which is the bottom of the reservoir.

The resulting flood wave would move down Prospect Hill, either toward Prospect Hill Pond if the break occurs on the south side of the reservoir or toward Black Pond if the break occurs on the north side. Water moving toward Prospect Hill Pond would probably damage Prospect Hill Street and could cause flooding of up to four residences southwest of the reservoir along Prospect Hill Street. The location of these structures is not shown on the Location Map since the buildings were built subsequent to the date of the map. Water moving toward Black Pond would not encounter any development.

## SECTION 6

### STRUCTURAL STABILITY

#### 6.1 Evaluation of Structural Stability

- a. Visual Observations. The evaluation of the structural stability of Prospect Hill Reservoir is based on review of available drawings and the visual inspection conducted on November 30, 1978. As discussed in Section 3, Visual Inspection, the dam is in good condition. No seepage or settlement of the embankment was observed, however, some trees are growing on the slopes. Also, it was reported that greater than anticipated seepage was occurring through the underdrain system. Based on this, the embankment is not considered to be a potential hazard.
- b. Design and Construction Data. There are 45 sheets of drawings dated September, 1954, and revised as-built in July, 1956, available from the Owner and Zoppo Construction Co. Copies of selected sheets are given in Appendix B (see Figures B-2 through B-6). A copy of the construction specifications and a set of construction photographs are available from Fay, Spofford & Thorndike, Inc. There are no structural or hydraulic computations available from the Owner, State, or County, relative to the design or construction of the reservoir and embankment.

Information does not appear to exist on the type, shear strength, and permeability of the soil and/or rock materials of the embankment. Borrow material for the earth embankment was reportedly sandy gravel obtained from cut sections around the reservoir. The foundation beneath the reservoir is also sandy gravel, and no bedrock was encountered. The embankment is unzoned earth fill with slopes at 2:1 (see typical section on Figure B-3). The embankment was placed in 1-foot layers and compacted mainly with tracked equipment (bull dozers). The project manager on the job recalls that compaction was at least 95 percent. All stones greater than 6 inches in

PROSPECT HILL RESERVOIR

length were removed prior to compaction. The embankment was constructed in the fall of 1955 and allowed to settle during the winter and spring. The downhill slope is covered with seeded loam, 1 foot thick. The slope into the reservoir is lined with reinforced concrete slabs, 6 inches thick.

- c. Operating Records. There is no instrumentation of any type in the embankment at Prospect Hill Reservoir, and no instrumentation was ever installed at this site. The performance of the embankment under prior loading can only be inferred by physical evidence at the site.
- d. Post-Construction Changes. Based on field measurements and discussions with personnel from Fay, Spofford & Thorndike, Inc. and Zoppo Construction Co., the embankment appears to be built essentially as shown on the contract drawings. No significant post-construction repairs have been made.
- e. Seismic Stability. The dam is located in Seismic Zone No. 2 and in accordance with Phase I "Recommended Guidelines" does not warrant seismic analyses.

## SECTION 7

### ASSESSMENT, RECOMMENDATIONS, AND REMEDIAL MEASURES

#### 7.1 Dam Assessment

- a. Condition. Based upon a review of available drawings, the visual inspection of the site and past performance, there is a need for maintenance and monitoring to assure the continued performance of this structure. Generally, the reservoir and embankment are considered to be in good condition. However, trees are growing on the slope of the embankment, slight leakage is occurring at some joints on conduits in the gate house, and a gate valve is reportedly leaking in the outlet conduit. Also greater than anticipated seepage is occurring in the underdrain system.

Hydraulic analyses indicate that the two 12-inch overflow drains can discharge a flow of 15.1 cfs with the water surface at El 194.0 which is the top of the rim of the reservoir. An outflow test flood of 7.7 cfs (one-half PMF) at El 192.9 will not overflow the reservoir under normal operating conditions.

- b. Adequacy. The lack of detailed design and construction data did not allow for a definitive review. Therefore, the evaluation of the adequacy of the reservoir and embankment is based primarily on review of available drawings, visual inspection, past performance and engineering judgment.
- c. Urgency. The remedial measures outlined below should be implemented by the Owner within 2 years after receipt of this Phase I Inspection Report.
- d. Need for Additional Investigation. Additional investigations to further assess the adequacy of the dam are not required at this time.

PROSPECT HILL RESERVOIR

- 7.2 Recommendations. As a result of the visual inspection and a review of available data, more detailed studies of the site are not considered necessary at the present time. Future changes in the reservoir structures or increases in seepage rates in the underdrain system may necessitate further investigations.

Recommendations on repairs and maintenance procedures are outlined below under Section 7.3, Remedial Measures.

7.3 Remedial Measures

- a. Operating and Maintenance Procedures. The dam and appurtenant structures are generally well maintained. However, it is recommended that the Owner accomplish the following:

- (1) selectively remove trees from the slopes of the earth embankment,
- (2) repair leakage from some joints on conduits in the gate house,
- (3) repair leakage from a gate valve on the outlet conduit,
- (4) monitor and measure the leakage from the underdrain system. Any changes should immediately be evaluated.
- (5) conduct periodic technical inspections of the reservoir and appurtenances every two years.

- 7.4 Alternatives. There are no alternatives to implementing the maintenance procedures listed above.

APPENDIX A  
PERIODIC INSPECTION CHECKLIST

PROSPECT HILL RESERVOIR



# PERIODIC INSPECTION

## PARTY ORGANIZATION

PROJECT PROSPECT HILL RES.

DATE NOV 30 1978

TIME 8:30 - 12:00

WEATHER Partly Sunny

W.S. ELEV. 192.0\* U.S. None D.N.S.

\*operational high water level given  
on the contract drawings

### PARTY:

- |                     |           |
|---------------------|-----------|
| 1. <u>R WEBER</u>   | 6. _____  |
| 2. <u>C SWEET</u>   | 7. _____  |
| 3. <u>H LORD</u>    | 8. _____  |
| 4. <u>D COLE</u>    | 9. _____  |
| 5. <u>W CHECCHI</u> | 10. _____ |

### PROJECT FEATURE

### INSPECTED BY

### REMARKS

- |                          |                         |  |
|--------------------------|-------------------------|--|
| 1. <u>EMBANKMENT</u>     | <u>WEBER</u>            |  |
| 2. <u>INLET / OUTLET</u> | <u>WEBER / BRANAGAN</u> |  |
| 3. _____                 |                         |  |
| 4. _____                 |                         |  |
| 5. _____                 |                         |  |
| 6. _____                 |                         |  |
| 7. _____                 |                         |  |
| 8. _____                 |                         |  |
| 9. _____                 |                         |  |
| 10. _____                |                         |  |

# PERIODIC INSPECTION CHECK LIST

PROJECT PROSPECT HILL RES DATE NOV 30 1978  
 PROJECT FEATURE DAM (RESERVOIR) NAME R. WEBER  
 DISCIPLINE GEOTECHNICAL NAME \_\_\_\_\_

AREA EVALUATED	CONDITION
<u>DIKE EMBANKMENT</u>	
Crest Elevation	varies from 192.9 to 193.2
Current Pool Elevation	192.0
Maximum Impoundment to Date	unknown
Surface Cracks	NONE VISIBLE
Pavement Condition	NO PAVEMENT
Movement or Settlement of Crest	NONE VISIBLE
Lateral Movement	NONE VISIBLE
Vertical Alignment	LEVEL
Horizontal Alignment	RECTANGULAR
Condition at Abutment and at Concrete Structures	GOOD - DIKE MEETS NATURAL GROUND AT CREST EL. IN SOME AREAS
Indications of Movement of Structural Items on Slopes	NONE VISIBLE
Trespassing on Slopes	ANIMALS AND VANDALS
Sloughing or Erosion of Slopes or Abutments	SOME MINOR SURFACE EROSION. FIR TREES PLANTED TO CHECK EROSION
Rock Slope Protection - Riprap Failures	CONCRETE LINER IN RESERVOIR
Unusual Movement or Cracking at or near Toes	NONE VISIBLE
Unusual Embankment or Downstream Seepage	NONE VISIBLE AT RESERVOIR
Piping or Boils	NONE VISIBLE
Foundation Drainage Features	UNDERDRAIN SYSTEM IN BOTTOM OF RESERVOIR
Toe Drains	NONE
Instrumentation System	NONE

# PERIODIC INSPECTION CHECK LIST

PROJECT PROSPECT HILL RES DATE Nov 30 1978  
 PROJECT FEATURE DAM (STORAGE BASIN) NAME R. WEBER  
 DISCIPLINE GEOTECHNICAL NAME \_\_\_\_\_

AREA EVALUATED	CONDITION
<u>DIKE EMBANKMENT</u>	
Crest Elevation	<i>varies from 135.5 to 136.0</i>
Current Pool Elevation	<i>135.5</i>
Maximum Impoundment to Date	<i>unknown</i>
Surface Cracks	<i>NONE VISIBLE</i>
Pavement Condition	<i>NO PAVEMENT</i>
Movement or Settlement of Crest	<i>EROSION OF CREST DUE TO OVER FLOW</i>
Lateral Movement	<i>NONE VISIBLE</i>
Vertical Alignment	<i>LEVEL</i>
Horizontal Alignment	<i>TRIANGULAR</i>
Condition at Abutment and at Concrete Structures	<i>NONE</i>
Indications of Movement of Structural Items on Slopes	<i>NONE VISIBLE</i>
Trespassing on Slopes	<i>UNKNOWN</i>
Sloughing or Erosion of Slopes or Abutments	<i>EROSION OF SLOPE DUE TO OVERTOPPING</i>
Rock Slope Protection - Riprap Failures	<i>NONE</i>
Unusual Movement or Cracking at or near Toes	<i>NONE VISIBLE</i>
Unusual Embankment or Downstream Seepage	<i>NONE VISIBLE</i>
Sliding or Boils	<i>NONE VISIBLE</i>
Foundation Drainage Features	<i>GRAVEL BOTTOM FOR PERCOLATION OF WATER</i>
Drains	<i>NONE</i>
Instrumentation System	<i>NONE</i>

# PERIODIC INSPECTION CHECK LIST

PROJECT Prospect Hill RES

DATE Nov 30 1978

PROJECT FEATURE OUTLET

NAME R. Weber

DISCIPLINE Geotechnical

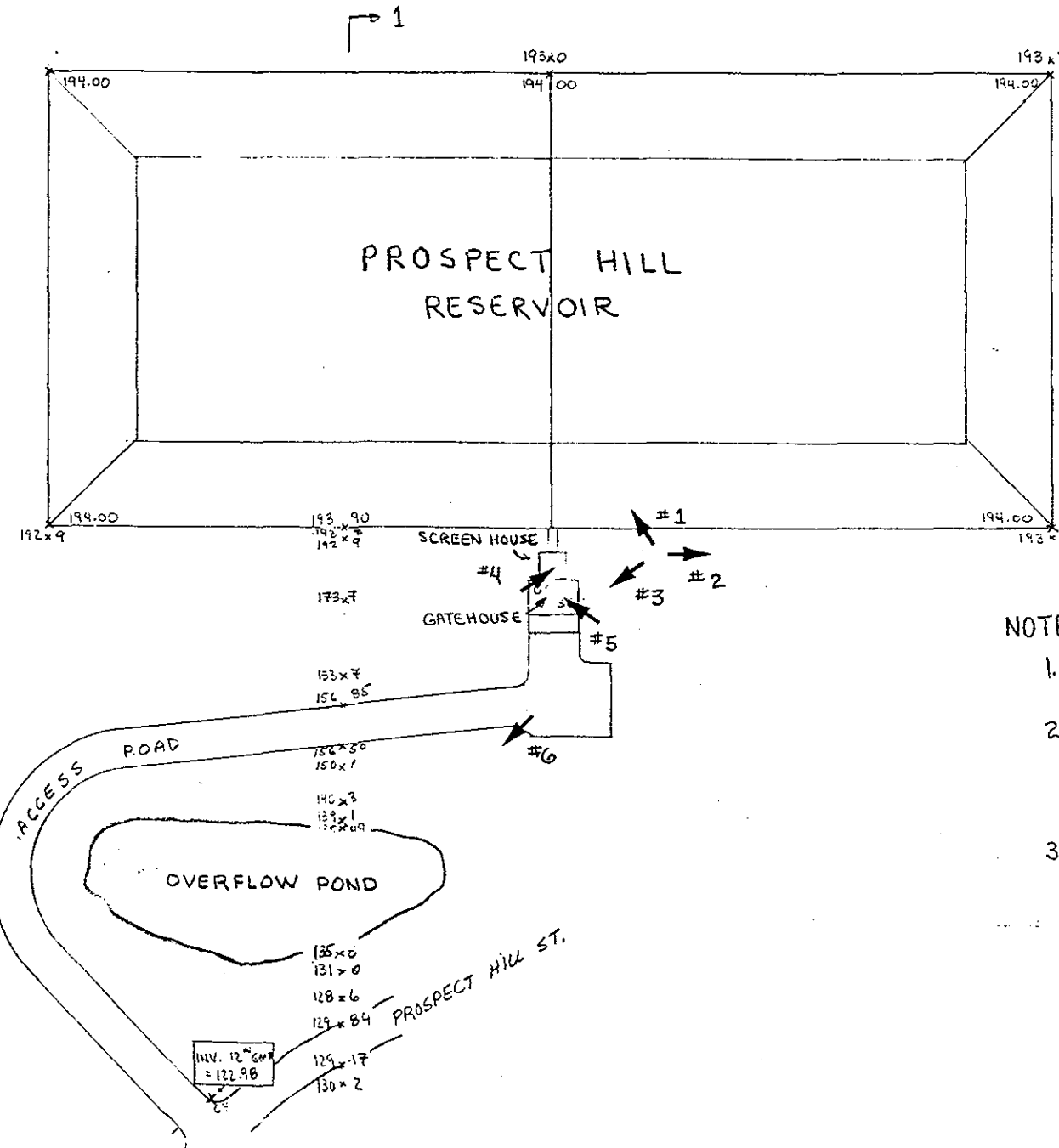
NAME L. Branagan

AREA EVALUATED	CONDITION
<u>OUTLET WORKS - CONTROL TOWER</u>	
a. Concrete and Structural	
General Condition	good
Condition of Joints	good
Spalling	none visible
Visible Reinforcing	none visible
Rusting or Staining of Concrete	STAINING due to LEAKS AT PIPES
Any Seepage or Efflorescence	SEEPAGE WHERE PIPES COME THROUGH WALL ALSO SOME EFFLORESCENCE
Joint Alignment	good
Unusual Seepage or Leaks in Gate	none visible
Cracks	FEW BUT VERY MINOR
Rusting or Corrosion of Steel	none visible
b. Mechanical and Electrical	
Air Vents	
Float Wells	
Crane Hoist	
Elevator	
Hydraulic System	
Service Gates	20-inch valves - hand operated
Emergency Gates	12-inch drains with valves
Lightning Protection System	
Emergency Power System	
Wiring and Lighting System in Gate Chamber	yes

APPENDIX B  
PLANS OF RESERVOIR

	<u>Page</u>
Figure B-1, Plan of Reservoir from field survey, November 30, 1978	B-1
Figures B-2 through B-6, Contract Drawings (selected sheets) dated September, 1954, revised as built July, 1956	
Site Plan	B-2
Longitudinal and Transverse Sections	B-3
Structural Details	B-4
Gate House - Plan Showing Piping	B-5
Gate House - Sections Showing Piping	B-6

189\*8

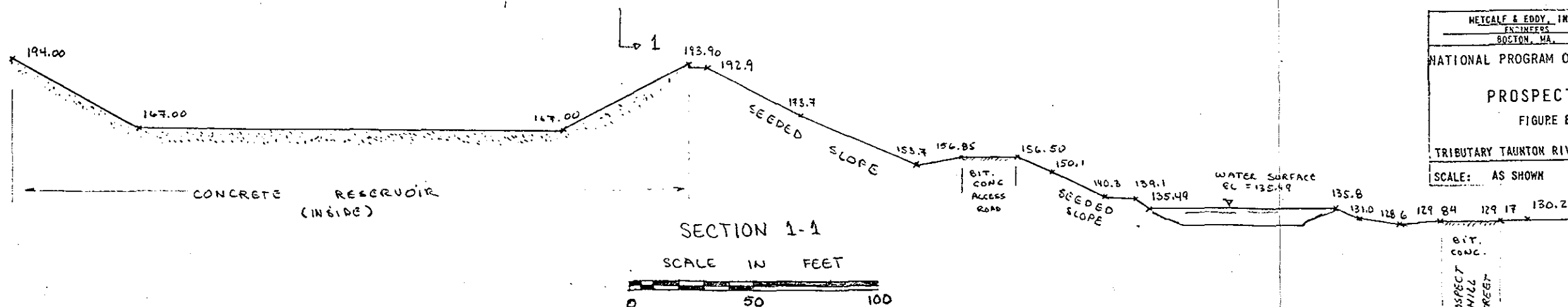


PLAN SCALE



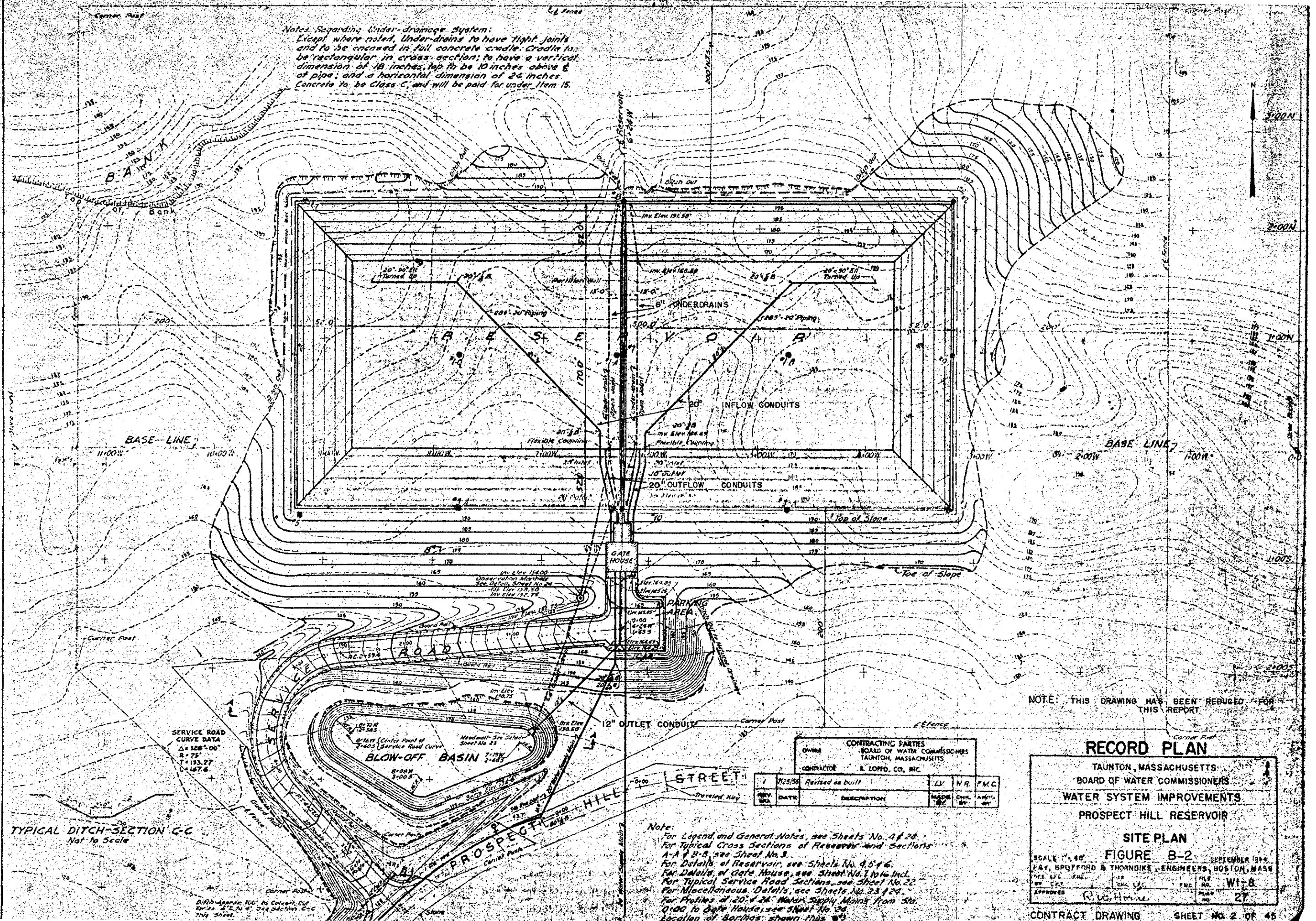
NOTES:

1. ELEVATIONS SHOWN ARE REFERENCED TO TOP CENTERWALL ELEV. 194.0 (MSL).
2. INFORMATION SHOWN BASED ON FIELD SURVEY OF NOVEMBER 30, 1978, AND CONTRACT DRAWINGS DATED SEPTEMBER, 1954.
3. #2 INDICATES LOCATION AND DIRECTION OF VIEW FOR PHOTOGRAPHS



MEYCALF & EDDY, INC. ENGINEERS BOSTON, MA.	U.S. ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MA.
NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS	
PROSPECT HILL RESERVOIR	
FIGURE B-1 PLAN OF RESERVOIR	
TRIBUTARY TAUNTON RIVER	MASSACHUSETTS
SCALE: AS SHOWN	DATE: JANUARY, 1979

Notes Regarding Under-drainage System:  
 Except where noted, Under-drains to have tight joints  
 and to be encased in full concrete cradle. Cradle to  
 be rectangular in cross section; to have a vertical  
 dimension of 18 inches, top to be 10 inches above  
 of pipe; and a horizontal dimension of 24 inches.  
 Concrete to be Class C, and will be paid for under Item 15.



NOTE: THIS DRAWING HAS BEEN REDUCED FOR THIS REPORT

CONTRACTING PARTIES			
BOARD OF WATER COMMISSIONERS TAUNTON, MASSACHUSETTS			
CONTRACTOR R. L. ZOPPO, CO., INC.			
REV.	DATE	DESCRIPTION	BY
1	12/25/50	Revised as built	LV
2			NR
3			PMC

**RECORD PLAN**

TAUNTON, MASSACHUSETTS  
 BOARD OF WATER COMMISSIONERS  
 WATER SYSTEM IMPROVEMENTS  
 PROSPECT HILL RESERVOIR

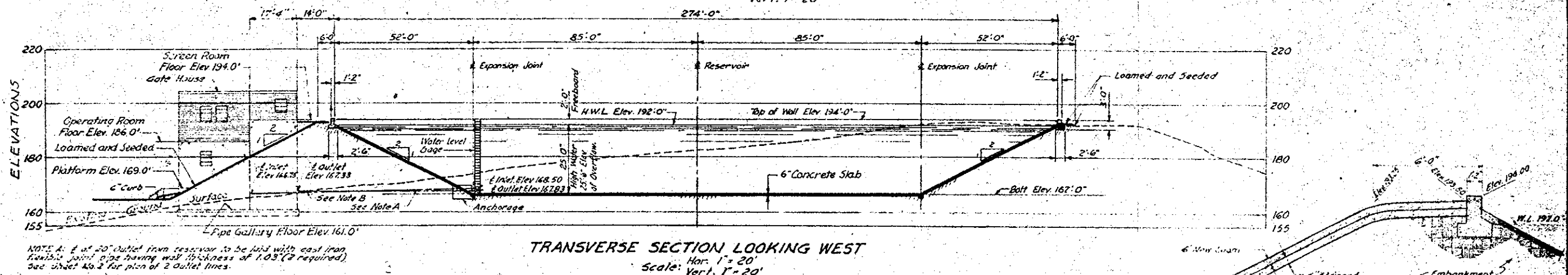
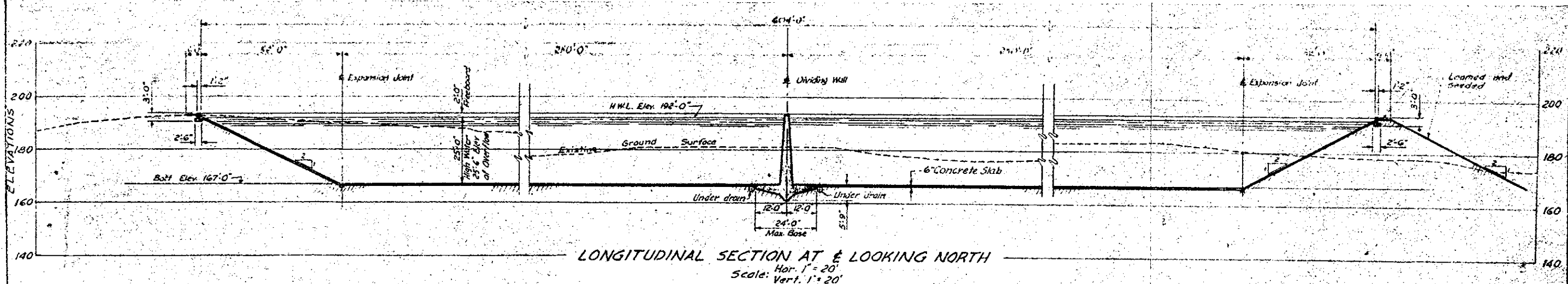
**SITE PLAN**

SCALE 1" = 40' **FIGURE B-2** SEPTEMBER 1954  
 FAY, SPURFORD & THORNDIKE ENGINEERS, BOSTON, MASS.

APPROVED: *R. L. Zoppo* **W-8**  
 27

CONTRACT DRAWING SHEET NO. 2 OF 45

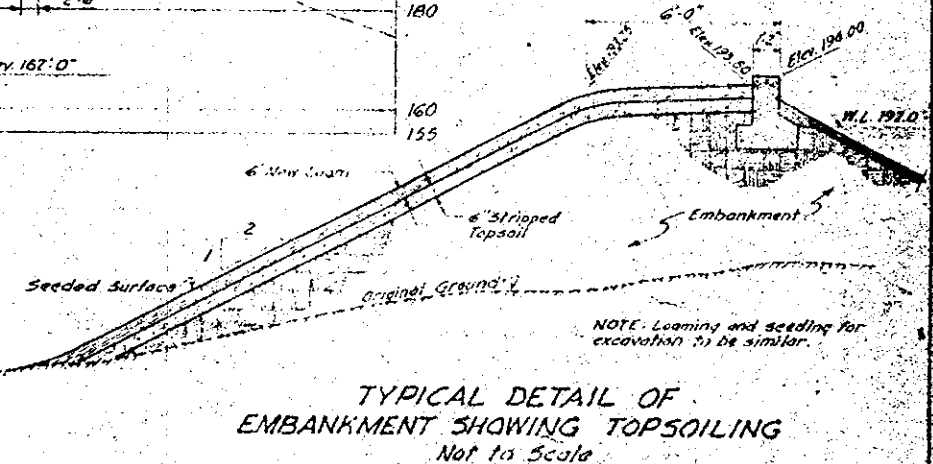
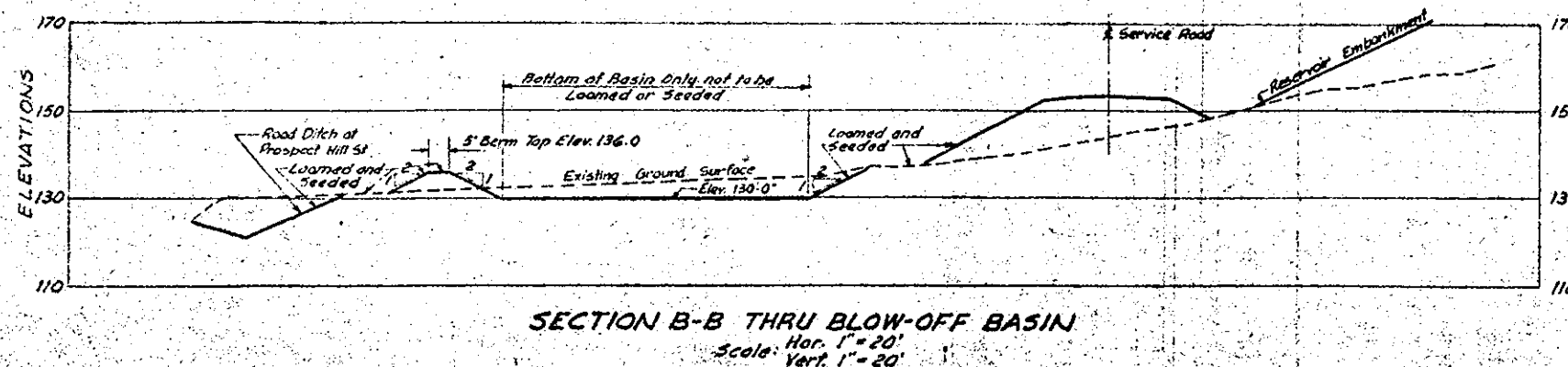
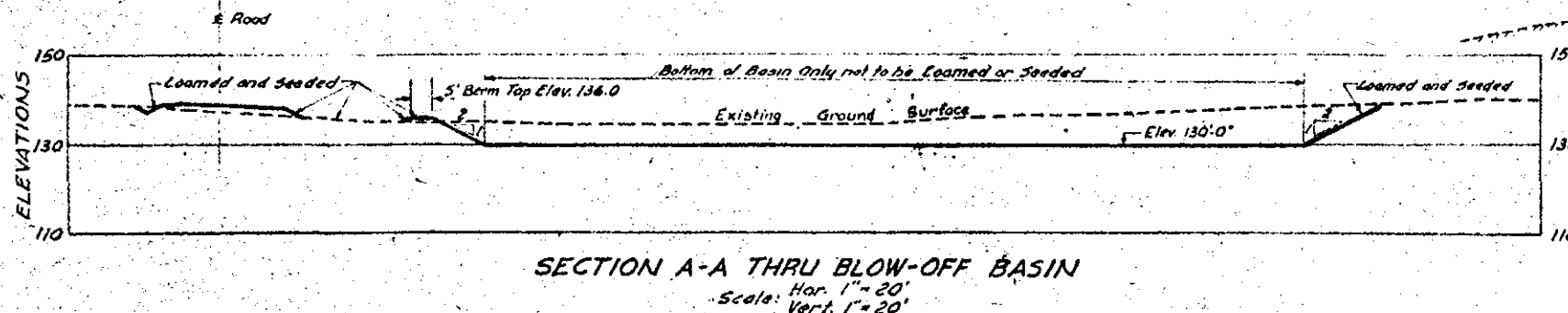
Note:  
 For Legend and General Notes, see Sheets No. 4 & 24.  
 For Typical Cross Sections of Reservoir and Sections A-A & B-B, see Sheet No. 3.  
 For Details of Reservoir, see Sheets No. 4, 5 & 6.  
 For Details of Gate House, see Sheet No. 7, 14 & 15.  
 For Typical Service Road Sections, see Sheet No. 22.  
 For Miscellaneous Details, see Sheets No. 23 & 24.  
 For Profiles of 20" V. 24" Water Supply Main from Sta. 0+00 to Gate House, see Sheet No. 24.  
 Location of Barriers shown thus: [Symbol]



NOTE A: 2" of 20" outlet from reservoir to be laid with cast iron flexible joint pipe having wall thickness of 1.03" (2 required). See Sheet No. 2 for plan of 2 outlet lines.

NOTE B: 2" of 20" inlet to reservoir to be laid with cast iron flexible joint pipe having wall thickness of 1.03" (2 required). See Sheet No. 2 for plan of 2 inlet lines.

See Sheets No. 6 and 14 for details of anchorages in Reservoir and of terminal points at Gate House.



NOTE: THIS DRAWING HAS BEEN REDUCED FOR THIS REPORT

DETAIL OF WATER LEVEL GAGE  
 Scale: 1" = 10'

RECORD PLAN

TAUNTON, MASSACHUSETTS  
 BOARD OF WATER COMMISSIONERS  
 WATER SYSTEM IMPROVEMENTS  
 PROSPECT HILL RESERVOIR

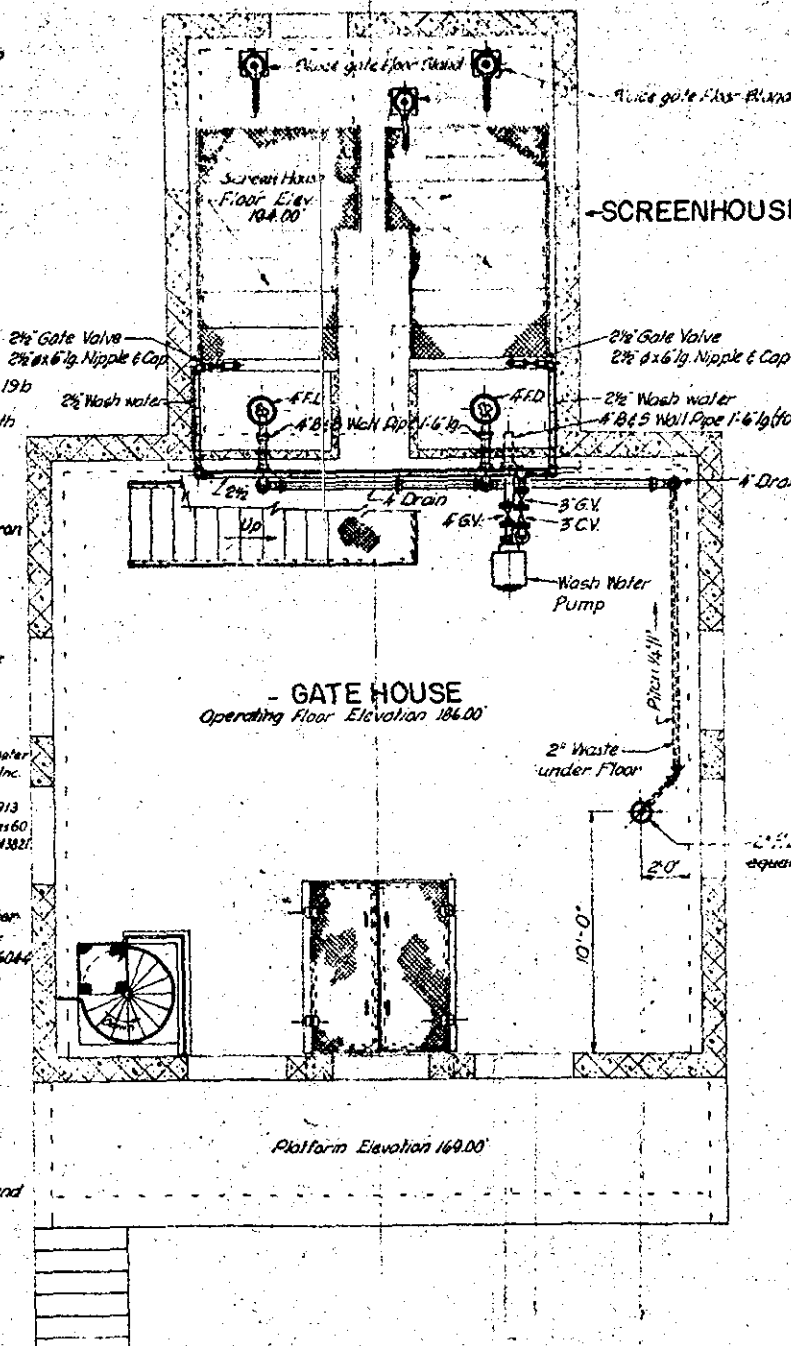
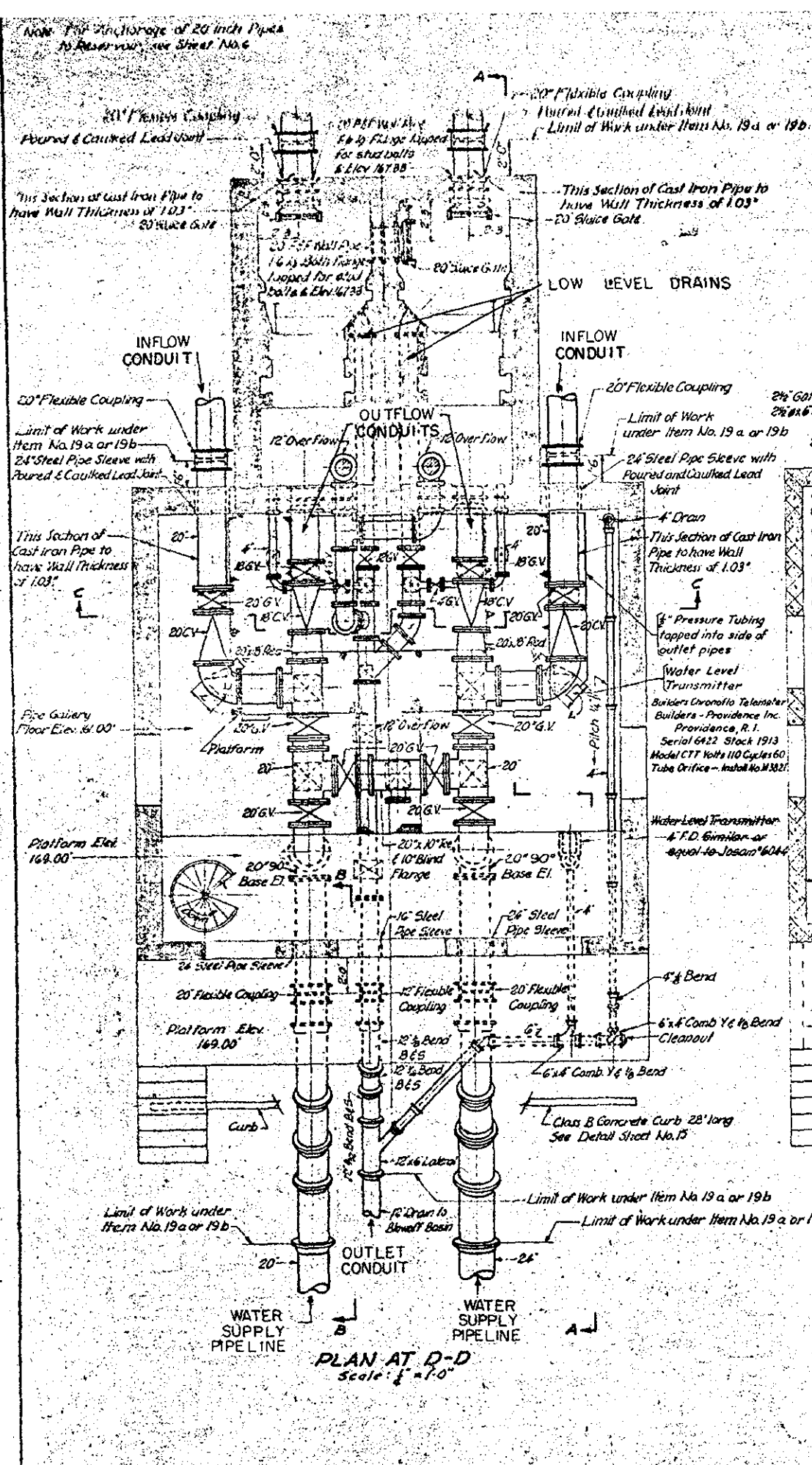
LONGITUDINAL & TRANSVERSE SECTIONS  
 FIGURE B-3  
 SEPTEMBER 1954  
 FAY, SPOFFORD & THORNOIKE, ENGINEERS, BOSTON, MASS.

DATE	BY	CHK	APP	FILE NO.	PLAN NO.
SEP 1954	W. B.	W. B.	W. B.	W-8	28

CONTRACT DRAWING SHEET NO. 3 OF 45

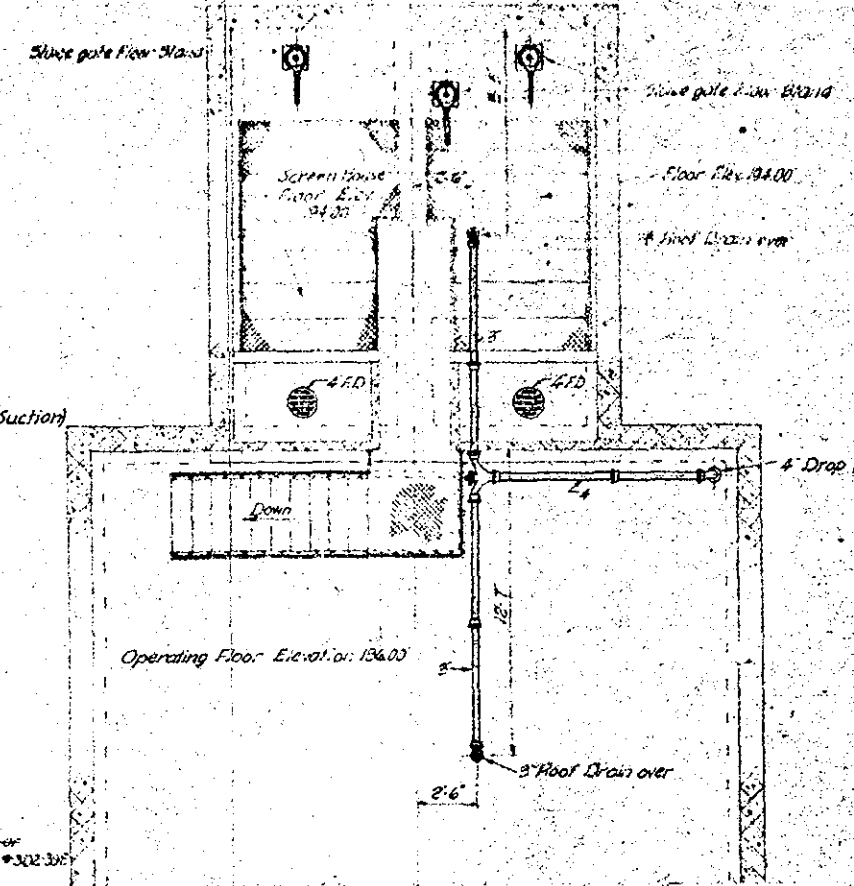
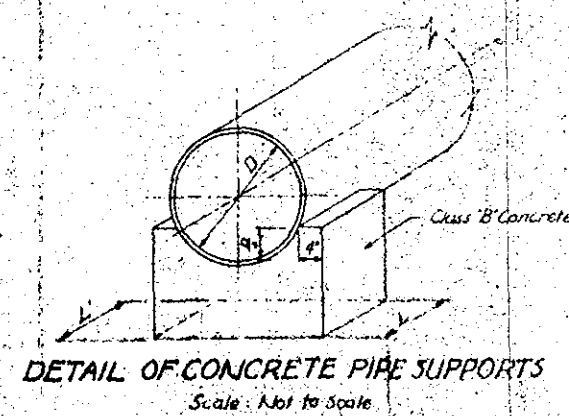






PLAN AT E-E  
Scale:  $\frac{1}{4}'' = 1'-0''$

Line Size	Location	L	L'
20"	Run of Pipe	1'-0"	1'-0"
20"	Tee	2'-2"	2'-2"
20"	Elbow	2'-0"	5"
30"	Gate Valve	10'	10"
18"	Gate Valve	9'	9"
12"	Run of Pipe	1'-0"	1'-0"
12"	Wye	1'-0"	1'-0"
12"	Tee	1'-0"	1'-0"
4"	Run of Pipe	1'-0"	1'-0"



PART PLAN AT F-F  
Scale:  $\frac{1}{4}" = 1'-0"$

OWNER	CONTRACTING PARTY
	BOARD OF WATER COMMISSIONERS
	TAUNTON, MASSACHUSETTS
CONTRACTOR	R. ZOPPO CO., INC.

NOTE: THIS DRAWING HAS BEEN  
REDUCED FOR THIS REPORT

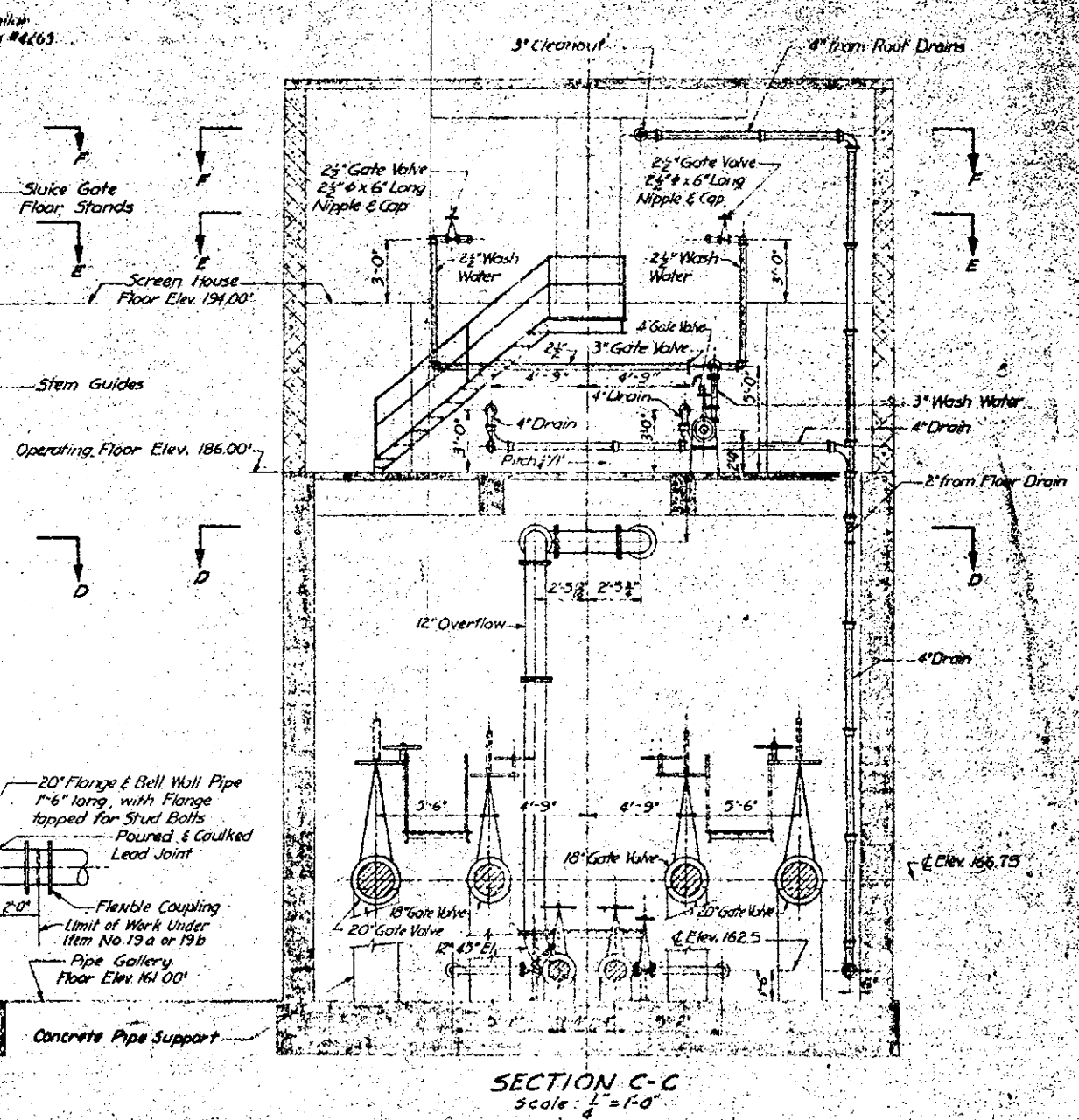
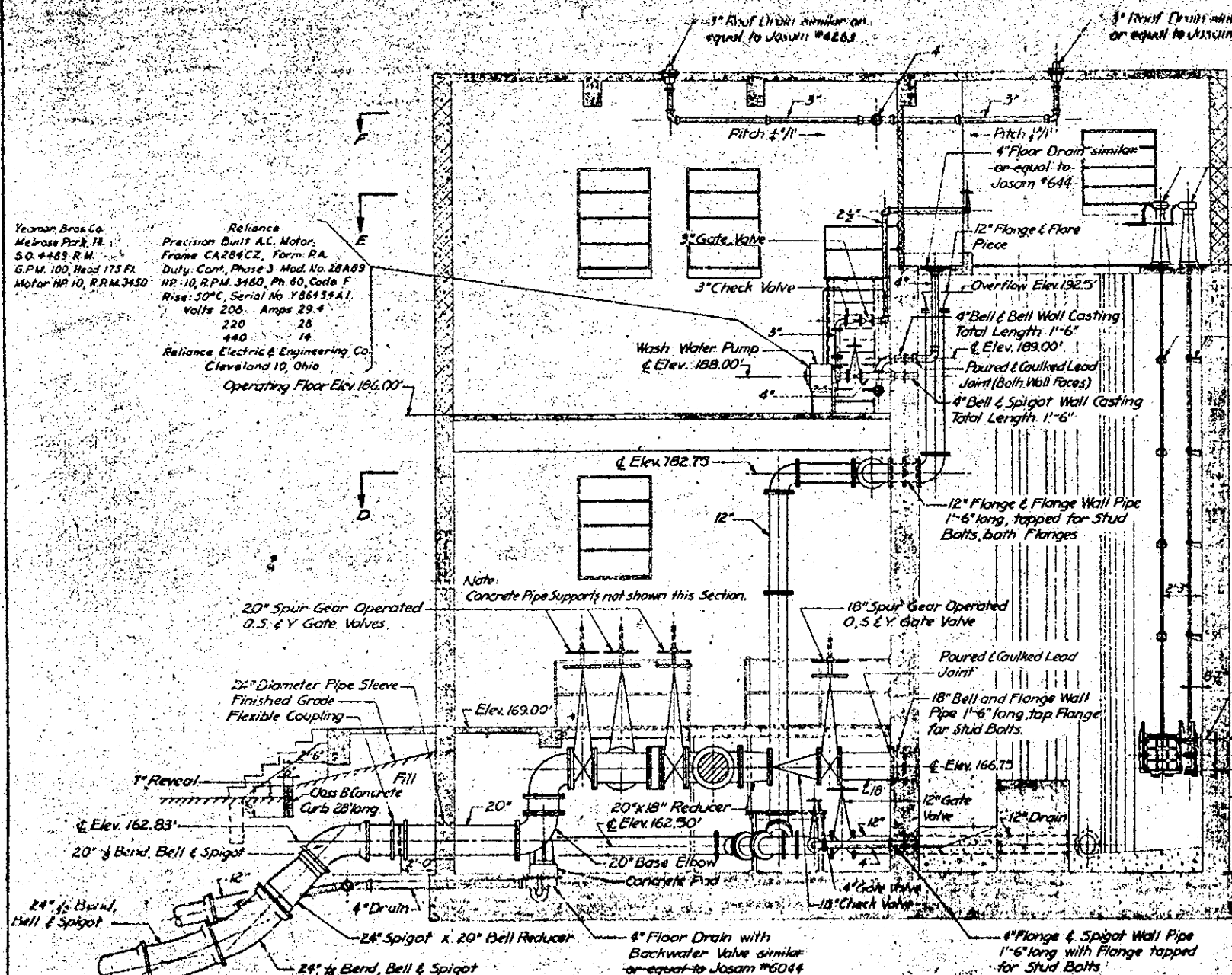
1	2056	Revised as built	LV	W.R	P.M.C
REV. NO.	DATE	DESCRIPTION	MADE BY	CNK BY	APPROV BY

## RECORD PLAN

TAUNTON, MASSACHUSETTS  
BOARD OF WATER COMMISSIONERS  
WATER SYSTEM IMPROVEMENTS  
PROSPECT HILL RESERVOIR  
GATE HOUSE  
PLAN SHOWING PIPING  
FIGURE B-5  
SCALE AS NOTED  
SEPTEMBER 1934  
JAY, SP. FIELD & THORNDIKE, ENGINEERS, BOSTON, MASS.  
DIN. ETC. ENG. JR. JOC. TINY. NO.  
DR. ETC. CON. ETC. YMC. NO.  
APPROVED  
Wt-8  
39

Yeoman Bros. Co.  
Melrose Park, Ill.  
S.O. 4489 R.M.  
G.P.M. 100, Head 175 Ft.  
Motor HP 10, R.R.M. 3450

Reliance  
Precision Built A.C. Motor  
Frame CA284CZ, Form PA  
Duty, Cont. Phase 3, Mod. No. 28A89  
HP 10, R.P.M. 3480, Ph. 60, Code F  
Rise: 50°C, Serial No. Y86634A.1  
Volts 208, Amps 29.4  
220 28  
440 14  
Reliance Electric & Engineering Co.  
Cleveland 10, Ohio  
Operating Floor Elev. 186.00'



Note:  
For Legend and General Notes, see Sheets No. 8 & 24.  
For Location of Sections A-A, B-B, C-C, Plan at D-D, E-E and Port Plan at F-F, see Sheet No. 12.

REV.	DATE	DESCRIPTION	MADE BY	CHECK BY	APPROVED BY
1	12/25/50	Revised as built			

## RECORD PLAN

TAUNTON, MASSACHUSETTS BOARD OF WATER COMMISSIONERS	
WATER SYSTEM IMPROVEMENTS	
PROSPECT HILL RESERVOIR GATE HOUSE	
SECTIONS SHOWING PIPING	
SCALE AS NOTED	FIGURE B-6 SEPTEMBER 1954
FAY, SPOFFORD & THORNDIKE, ENGINEERS, BOSTON, MASS.	
DES. LEC. P.M.C.	TR. J.C.C.
CHK. J.C.C.	CHK. L.F.C.
APPROVED	R.W. Horne
FILE NO.	WT-8
PLAN NO.	40

NOTE: THIS DRAWING HAS BEEN REDUCED FOR THIS REPORT

CONTRACTING PARTIES	
OWNER:	BOARD OF WATER COMMISSIONERS TAUNTON, MASSACHUSETTS
CONTRACT NO.	42340 CO. 44

APPENDIX C  
PHOTOGRAPHS

PROSPECT HILL RESERVOIR





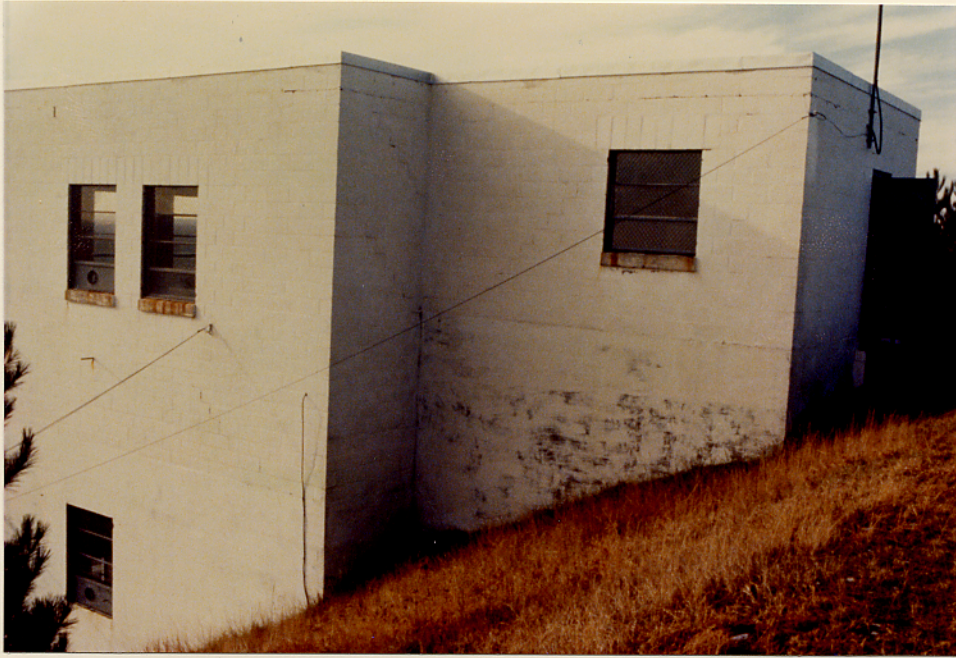
**NO. 1 VIEW OF CONCRETE WALLS OF RESERVOIR**



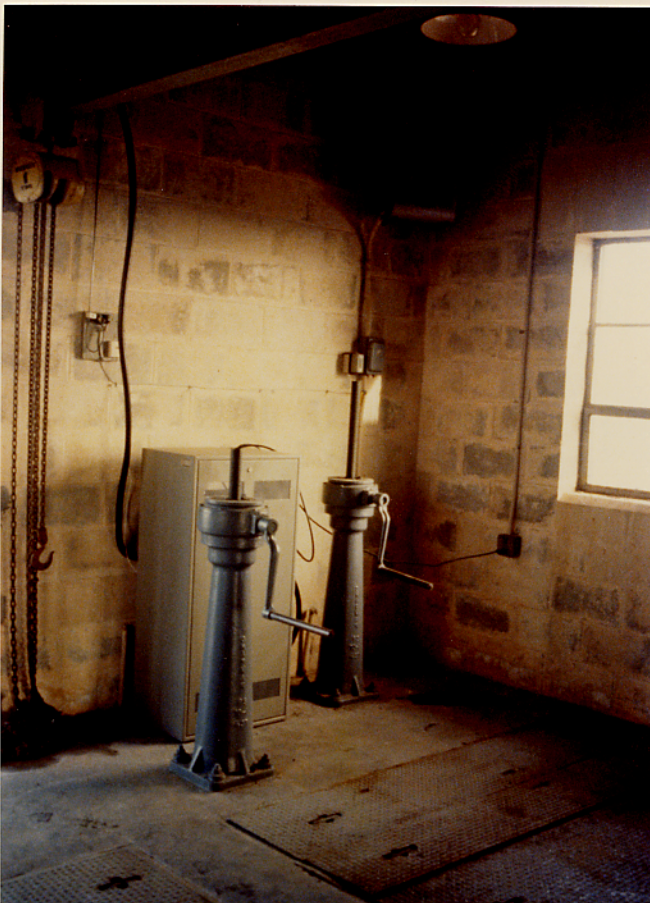
**NO. 2 VIEW OF CREST AND SLOPE OF EMBANKMENT**

PROSPECT HILL RESERVOIR





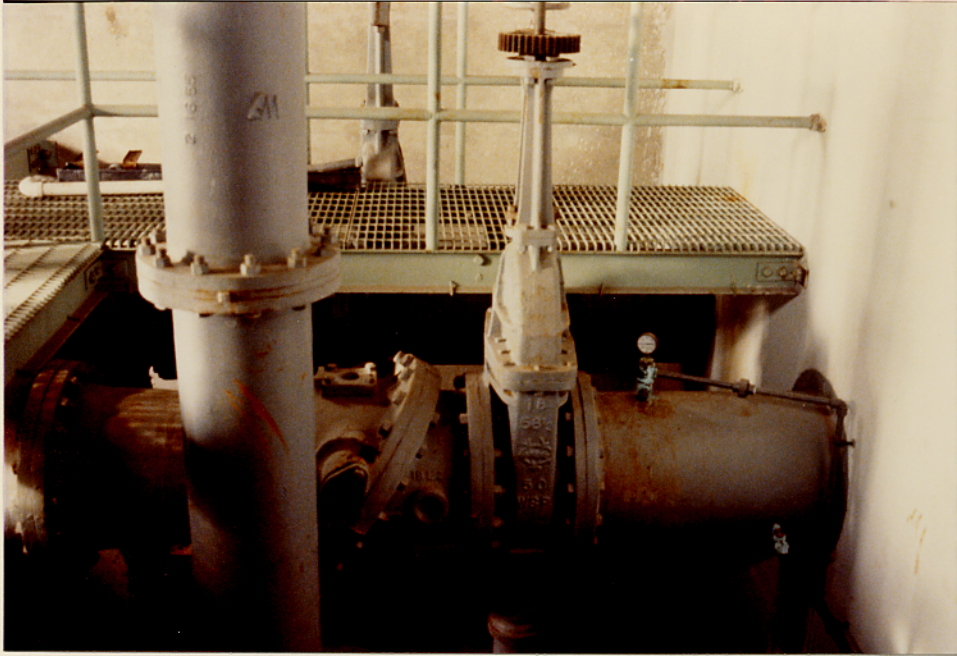
**NO. 3 VIEW OF GATE HOUSE ON SOUTH SIDE OF RESERVOIR**



**NO. 4 VIEW OF OPERATING MECHANISMS FOR SLIDE GATES**

PROSPECT HILL RESERVOIR





**NO. 5 VIEW OF OUTFLOW PIPE SHOWING  
GATE CONTROL AND CHECK VALVE**



**NO. 6 VIEW OF OVERFLOW BASIN**

PROSPECT HILL RESERVOIR

APPENDIX D  
HYDROLOGIC AND HYDRAULIC  
COMPUTATIONS

PROSPECT HILL RESERVOIR



# I Test Flood

## A. Rainfall Inflow (6 hr. storm)

Hour Ending	Full P.M.P. (Prob. Max. Precip.)		Half P.M.P.
	Increm. Rain (in.)	Rain Rate in/hr.	Increm. Rain (in.)
1	1.52	1.52	0.76
2	1.71	1.71	0.86
3	1.90	1.90	0.95
3:30	3.61	7.22	1.81
4	5.70	11.40	2.85
5	3.04	3.04	1.52
6	1.52	1.52	0.76

Size : Small ; Hazard : Significant ; Test Flood : 100yr to 1/2 hr

Use 1/2 PMF - for rain inflow without losses this  
is same as 1/2 P.M.P.

## B- Pumped Inflow

Taunton Water Dept. data indicates that the Prospect Hill Res. was supplied by 1- 5.5 m.g.d. pump and 1- 3.5 m.g.d. pump, at the most. (Other pumps primarily serve other areas)

∴ Max Pumped Inflow = 9 m.g.d. = 13.9 cfs. or 1.15 ac.ft./hr  
 For 6 hr. rainfall period, total vol. = 6.90 ac. ft.

## II Discharge Ratings

### A - Outlet Pipe

2- Vert. 12"  $\phi$  outlets w/ent. flared to 18"  $\pm$ , Ent. cr. Lvl - Say  $C_D \approx 0.98$

$$Q_0 = A_0 C_D \sqrt{2g H_0} = 2(1.785)(.98) \sqrt{64.4 H_0} = 12.34 H_0^{1/2}$$

Pipe Crest @ El. 192.5, Perim. Wall @ El. 194 ( $\pm$ )

Res El.	192.75	193	193.5	194	194.05	194.07
$H_0$	0.25	0.5	1.0	1.5	1.55	1.57
$Q_0$	6.2	8.7	12.34	15.1	15.4	15.5
ac. ft/hr	0.51	0.72	1.02	1.25	1.28	1.28
inches/hr	1.61	2.27	3.22	3.94	in 3.8 acre reservoir only	

### B - Crest Flow (Perimeter Wall)

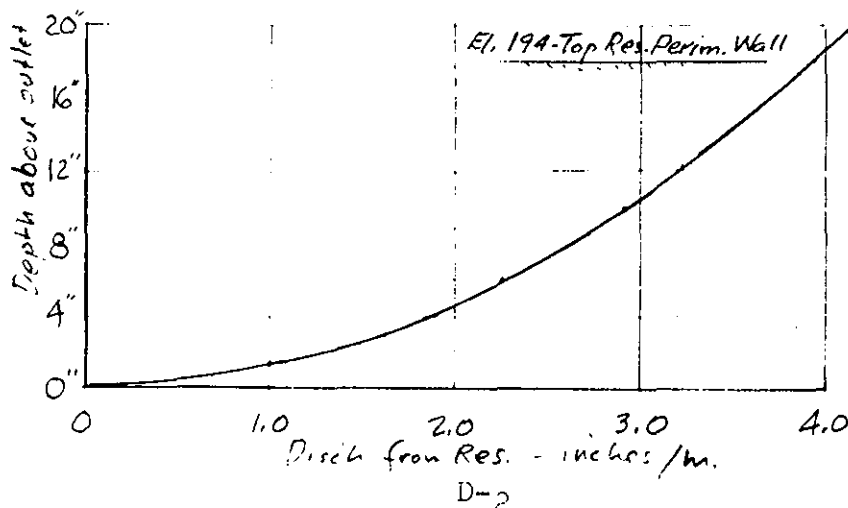
Res. area is 604' x 274' - Total length = 1756 ft  
 Assume 50% of length is effective, Crest El. 194  $\pm$

Use  $q = 2.55 H_c^{1.5}$  [Ref: V.T. Chow "Op. Chan. Hydr" pg 52]

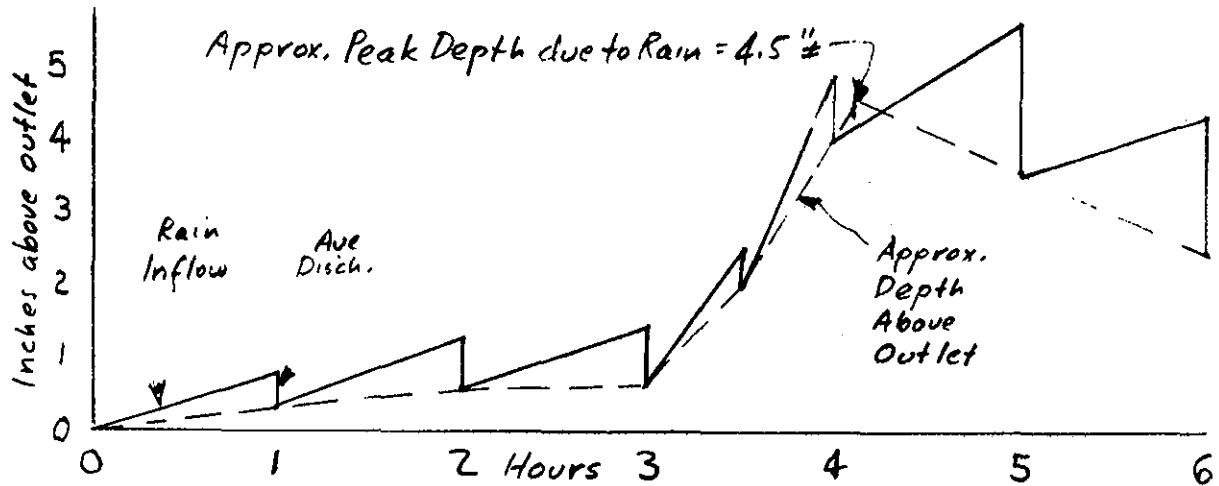
$$\therefore Q_c = 2239 H_c^{1.5}$$

Res. El.	194.02	194.05	194.07
$Q_c$	6.33	25.03	41.47

### C - Plot of Outlet Pipe Disch. vs. Head



### III Reservoir Storage



Time	Inches of Rain/in (1/2 PMP)	Ave Depth (in.)	Ave Disch (in.)	Final Depth (in.)
0 → 1 hr.	0.76	0.38	0.4	0.36
1 → 2 hr.	0.86	0.79	0.7	0.52
2 → 3 hr.	0.95	1.00	0.8	0.67
3 → 3.5 hr.	1.81	1.58	0.5	1.98
3.5 → 4.0 hr.	2.85	3.40	0.85	3.98
4 → 5 hr.	1.52	4.74	2.0	3.50
5 → 6 hr.	0.76	3.88	1.8	2.46

$\Sigma = 9.51$

$\Sigma = 7.05$

Max. Depth for Excess Pumping =  $18" - 4.5" = 13.5"$  - over 4.1 hours

Vol. =  $3.8 \text{ ac.} \left( \frac{13.5}{12} \right) = 4.275 \text{ ac. ft.} \div 4.1 = 1.04 \text{ ac. ft./hr.}$

$1.04 \text{ ac. ft./hr.} = 12.58 \text{ c.f.s.} = 8.1 \text{ m.g.d.} < 9 \text{ m.g.d. max. pump inflow}$

#### ④ Failure of Dam

##### Peak Failure Flow:

Pond Elevation - 194.0 (Top Wall)  
 Toe Elevation - 167.0 (Bot. of Res.)

$$Y_0 = 27.0$$

Dam Length Subject to Breaching = 250'

$$W_0 = 40\%(250) = 100$$

$$Q_R = 1.68 W_0 (Y_0)^{1.5} = 1.68(100)(27)^{1.5} = \underline{23,600 \text{ cfs}}$$

##### Storage Volume Released:

Storage Above Spillway N/A

Storage Below Spillway  $[250(170) + 302(274)]27 = 77.6 \text{ ac. ft.}$

S = Total Storage = 77.6 " "

##### Channel Hydraulics:

There is no channel.

##### Time to Drain:

$$\frac{43560 (77.6)}{3600 (1/2) (23600)} = 0.08 \text{ Hours, or 4.8 Minutes}$$

APPENDIX E

INFORMATION AS CONTAINED IN THE  
NATIONAL INVENTORY OF DAMS

PROSPECT HILL RESERVOIR

# REPORT OF DAMS IN THE UNITED STATES

①	②	③	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫	
STATE	IDENTITY NUMBER	DIVISION	STATE	COUNTY	CONGR. DIST.	STATE	COUNTY	CONGR. DIST.	NAME	LATITUDE (NORTH)	LONGITUDE (WEST)	REPORT DATE DAY   MO   YR
MA	111	FD	MA	DDN	10				PROSPECT HILL RESERVOIR	41°56.3	71°05.9	31 JAN 79

⑬	⑭
POPULAR NAME	NAME OF IMPOUNDMENT

⑮	⑯	⑰	⑱	⑲
REGION/CASIN	RIVER OR STREAM	NEAREST DOWNSTREAM CITY-TOWN-VILLAGE	DIST. FROM DAM (MI.)	POPULATION
01	NONE	TAUNTON	0	42100

⑳	㉑	㉒	㉓	㉔	㉕	㉖
TYPE OF DAM	YEAR COMPLETED	PURPOSES	STRUCTURAL HEIGHT (FT.)	HYDRAULIC HEIGHT (FT.)	IMPOUNDING CAPACITIES	
					MAXIMUM (ACRE-FT.)	NORMAL (ACRE-FT.)
RECIPG	1956	S	25	25	77	70

DIST OWN FED R PRV/FED SCS A VER/DATE  
 MED N N N N 21 FEB 79

㉗
REMARKS

㉘	㉙	㉚	㉛	㉜	㉝	㉞	㉟	㊱	㊲	㊳	㊴	㊵	㊶	㊷	㊸	㊹	㊺
O/S HAS	SPILLWAY			MAXIMUM DISCHARGE (FT.)	VOLUME OF DAM (CY)	POWER CAPACITY		NAVIGATION LOCKS									
	CREST LENGTH	TYPE	WIDTH (FT.)			INSTALLED (MW)	PROPOSED (MW)	NO	LENGTH (FT.)	WIDTH (FT.)	LENGTH (FT.)	WIDTH (FT.)	LENGTH (FT.)	WIDTH (FT.)	LENGTH (FT.)	WIDTH (FT.)	
2					16000												

㉡	㉢	㉣
OWNER	ENGINEERING BY	CONSTRUCTION BY
TAUNTON WATER WORKS	FAY SPOFFORD + THORNDIKE	R ZIPPEN CONSTRUCTION CO

㉤ REGULATORY AGENCY			
DESIGN	CONSTRUCTION	OPERATION	MAINTENANCE
NONE	NONE	NONE	NONE

㉥	㉦	㉧
INSPECTION BY	INSPECTION DATE DAY   MO   YR	AUTHORITY FOR INSPECTION
PRICALE AND EDDY INC	30 SEP 78	PUBLIC LAW 92-327

㉨
REMARKS